

## **TEMPORARY DRAINAGE PROPOSAL**

Proposed Temporary Shop and Services for a  
Period of 3 Years at Lot 3927 S.A ss.4 (Part)  
in D.D. 124 ,Tan Kwai Tsuen, Hung Shui Kiu,  
Yuen Long,New Territories

Jul 2026

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## 2 Introduction

### 2.1 Background

This report presents the drainage proposal for supporting the proposed temporary shop and services for a period of 3 years at Lot 3927 S.A ss.4 (Part) in D.D. 124, Tan Kwai Tsuen, Hung Shui Kiu, Yuen Long.

### 2.2 Objectives of the Report

This report shall be prepared to include the following:

- Identify the potential drainage impact assessment from the proposed Application Site
- Recommend and implement all necessary measures to mitigate adverse drainage impacts arising from the application site

### 2.3 Report Structure

The report contains the following sections:

- Section 1 on Introduction;
- Section 2 on Development Proposal;
- Section 3 on Assessment Criteria;
- Section 4 on Potential Drainage Impact; and
- Section 5 on Conclusion.

## 3 Development Proposal

### 3.1 Location of the Application Site

The application Site is located within the Tan Kwai Tsuen, Hung Shui Kiu, Yuen Long, with an area of around 340m<sup>2</sup> and ground level varying between + 26.5mPD and + 26.3mPD. The layout plan is provided in **Appendix B**.

This application site is "Residential (Group) 1" zoning, the type of application is the Temporary Use/Development in Rural Areas for a Period of 3 Years.

There is an existing village drain nearby the application site, which the runoff from the application site will be conveyed to, the drainage layout plan is attached in **Appendix C**.

## 4 Assessment Criteria

### 4.1 Design Return Periods

The drainage system in the Application site is to collect surface flows and convey to the terminate

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catchpit and finally convey to downstream village drain. The recommended design return periods based on the flood levels for the various drainage systems depend on the drainage system, land use, hazard to public safety and community expectations. The recommended design return period is reproduced in Table 4-1 below:

| DESCRIPTION                                                               | DESIGN RETURN PERIODS |
|---------------------------------------------------------------------------|-----------------------|
| Intensively Used Agricultural Land                                        | 2 – 5 Years           |
| Village Drainage including internal Drainage System under a polder Scheme | 10 Years              |
| Main Rural Catchment Drainage Channels                                    | 50 Years              |
| Urban Drainage Trunk System                                               | 200 Years             |
| Urban Drainage Branch System                                              | 50 Years              |

Table 4-1 Recommended Design Return Periods based on Flood Levels

As per Storm Drainage Manuel (SDM) Section 6.6.2 Urban Drainage Branch and Urban Drainage Trunk Systems “An ‘Urban Drainage Branch System’ is defined as a group or network of connecting drains collecting runoff from the urban area and conveying stormwater to a trunk drain, river or sea. For a simple definition, the largest pipe size or the equivalent diameter in case of a box culvert in a branch system will normally be less than 1.8m.

An ‘Urban Drainage Trunk System’ collects stormwater from branch drains and/or river inlets, and conveys the flow to outfalls in river or sea. Pipes with size or diameter equal to or larger than 1.8m are normally considered as trunk drains.”

As per SDM, since the proposed U-channels are sized smaller than 1.8m, the drainage system would be defined as an urban drainage branch with recommended design return period of 50 years.

The 50 years design return period will be considered to ensure adequacy of the stormwater drainage system.

## 4.2 Calculation Methodology for Runoff

Peak instantaneous runoff values before and after the development were calculated based on the Rational Method and with recommended physical parameters including runoff coefficient (C) and storm constants for different return periods referred to the SDM, based on the following equation:

$$Q_p = 0.278 C i A$$

|       |       |   |                                 |
|-------|-------|---|---------------------------------|
| where | $Q_p$ | = | Peak Runoff, m <sup>3</sup> /s  |
|       | C     | = | Runoff Coefficient              |
|       | i     | = | Rainfall Intensity, mm/hr       |
|       | A     | = | Catchment Area, km <sup>2</sup> |

The paved area of the site will account for 5240m<sup>2</sup>. For conservative, the runoff coefficient of 0.9 is assumed, such that the all the run-off would be collected from the catchment area without any infiltration as the critical scenario.

Based on the storm constants for 50-year return period recommended in the SDM, the appropriate rainfall intensities (i) are calculated as detailed in Appendix D

## 4.3 Calculation Methodology for Pipe Capacity Checking

Due to the catchment areas are less than 1ha, U-channels are recommended to be constructed to collect the stormwater runoff of the open area within the site. For the catchment area within the roofing of the one-story warehouse, stormwater would be collected by the gutter, and then be diverted to U-channel system at ground level via downpipe. The collected stormwater should finally be diverted to the downstream via the proposed U-channel system.

For the worst-case scenario, bad condition of concrete pipe is assumed for the Manning's roughness coefficient (coefficient value is 0.016) for calculating capacities of concrete U-channel using Manning's Equation.

Manning's Equation for calculating the channel and pipe capacities is adopted.

# 5 Potential Drainage Impact

## 5.1 Existing Site Condition

The application Site is located within the Tan Kwai Tsuen, Hung Shui Kiu, Yuen Long with an area of around 340m<sup>2</sup> and ground level varying between + 26.5mPD and + 26.3mPD.

## 5.2 Changes in Drainage Characteristics

Since the existing ground level of application site is generally higher than the adjacent ground surface. No external catchment shall be considered in the calculation. And the hoarding of adjacent site control the runoff within application site won't convey to adjacent site, prevent extra runoff affecting adjacent site.

The change in sub-catchment is summarized in Table 5-1.

|                                          | Before | After |
|------------------------------------------|--------|-------|
| Grassland (m <sup>2</sup> )              | 0      | 0     |
| Paved Area (m <sup>2</sup> )             | 340    | 340   |
| External Catchment Area(m <sup>2</sup> ) | 0      | 0     |
| Total Catchment Area (m <sup>2</sup> )   | 340    | 340   |

Table 5-1 Change in sub-catchment within the site

## 5.3 Potential Drainage Impact

The details of the proposed drainage works are illustrated in **Appendix C**.

To effectively convey stormwater away from the application site and minimize the potential impact to the drainage infrastructure of the village area, drainage works consists of U-channels, are proposed to convey the stormwater runoff to the terminate catchpit with sand trap (TCP) within the site.

The runoff within application site separated to three catchment area, catchment area CA1 approximate 130m<sup>2</sup>, collected by a 150mm U-channel, runoff would then be conveyed to downstream village drain. Catchment area CA2 approximate 210m<sup>2</sup> is collected by one 150mm U-channel along the lower-level boundary, runoff would then be conveyed to terminate catchpit, and then be conveyed to downstream village drain. For conservative, a 150mm U-channel located at middle of application site to convey the runoff to the terminate catchpit within the application site, and eventually discharge to the further downstream as indicated in the **Appendix C**.

The U-channels receive stormwater from the surface. The design calculation of the proposed drainage is provided in **Appendix D**. The design calculation is summarized in Table 4-2.

| Drainage System        | Estimated Flow<br>(L/min) | Capacity<br>(L/min) | Reserve Capacity |
|------------------------|---------------------------|---------------------|------------------|
| 150mm u-channel (CA1)  | 761                       | 1500                | 49%              |
| 150 mm u-channel (CA2) | 1213                      | 1600                | 19%              |

Table 4-2 Design calculation of the proposed drainage work

1. Rainfall increase due to climate change at the end of 21st century is considered according to stormwater drainage manual Table 28.

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2. The reserve capacity is calculated by assuming that the U-channel reach its full capacity.

The design runoff arise from the proposed Application Site is to be discharged into the proposed existing terminate catchpits with the runoff anticipated to be 761L/min and 1213L/min, which is within the drainage capacity of the proposed 150mm u-channel of 1600L/min, the reserve capacity is 19%.

It is considered that the drainage discharge from the Application Site will not cause adverse impact to the entire downstream drainage system.

All u-channels & catch pits will be constructed according to the CEDD's standard drawings, please refer to the **Appendix E**.

## 6 Construction Stage

### 6.1 Temporary Drainage Arrangements

Proper measures shall be taken to maintain the existing drainage characteristics of the catchment areas and to minimize drainage impacts associated with the construction works. The principal drainage impacts which are associated with construction of the works have been identified as follows:

- (i) Erosion of ground materials;
- (ii) Sediment transportation to existing downstream drainage system; and
- (iii) Obstruction to drainage systems.

Regular inspections shall be carried out to ensure integrity of the works. These inspections shall cover works under construction as well as recently completed areas.

To ensure proper operation of the site drainage channels and desilting facilities, inspection of the perimeter drains shall be carried out on a weekly basis and the desilting facilities shall be cleaned on a daily basis.

If excavated materials are not possible to transport away the excavated material within the same day, the material should be covered by tarpaulin/impervious sheets. Stockpiles of construction materials (for examples aggregate, fill materials) of more than 50 m3 in an open area shall also be covered with tarpaulin or similar fabric during rainstorms.

All runoff discharged into the existing drainage system will be settled in a silt trap to ensure no sediment will be discharged into the channel. Silt traps will normally be provided along the site drainage

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immediately upstream of the proposed discharge point to the existing Site. The silt traps will be inspected daily and immediately after each rainstorm.

Liaison will be carried out with relevant parties regarding temporary drainage arrangements to ensure that the drainage system is functioning adequately.

## 7 Conclusions

### 7.1 Conclusions

The analyzed catchment area of 340m<sup>2</sup> consists of the site area of the proposed Application Site only and no external catchment area had been identified.

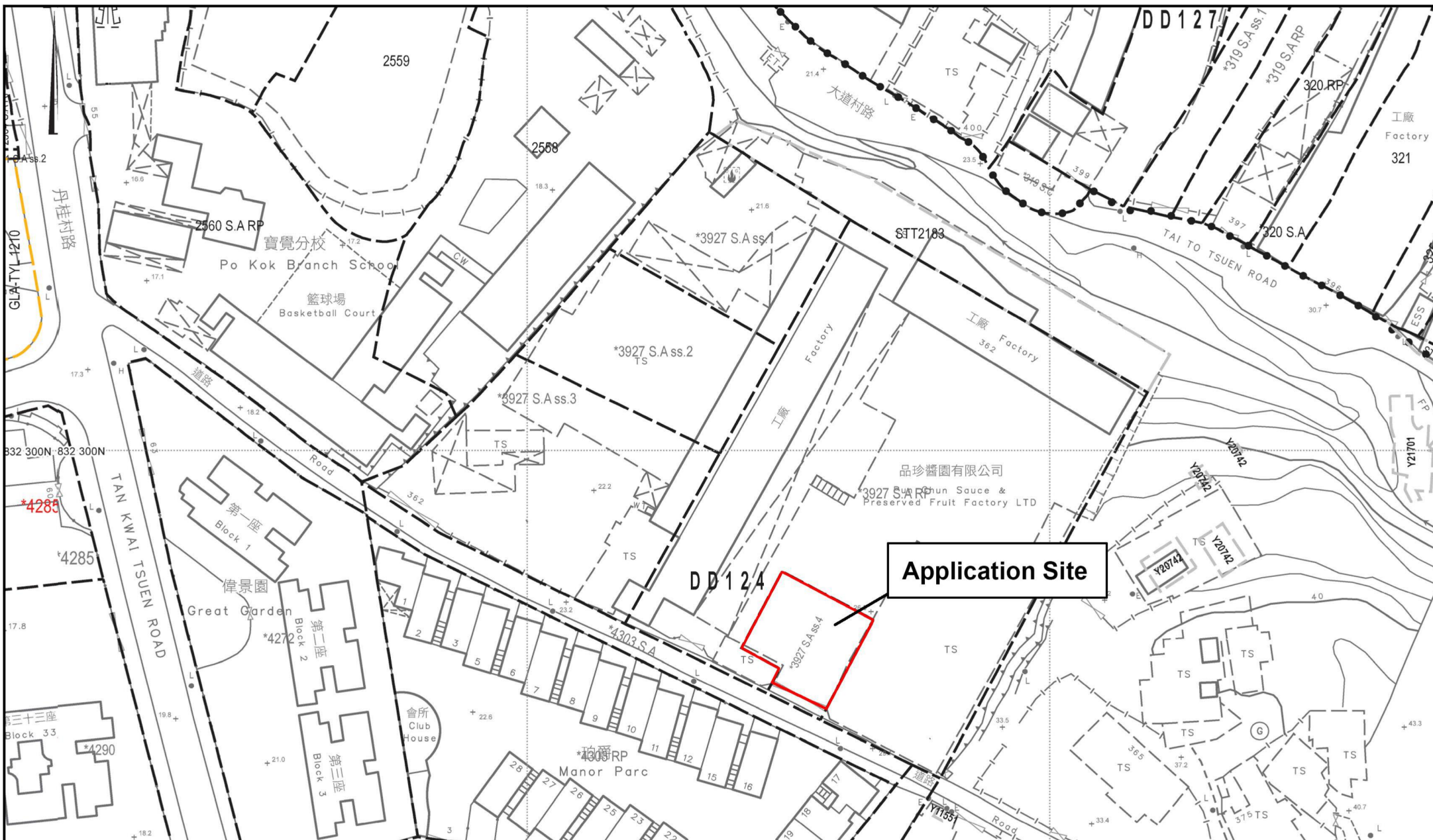
U-channels are proposed to convey runoff from the application site for collection. The proposed U-channels are located along the lower-level boundary which is subject to change to suit the building layout.

The assessment reviews the drainage pipe have the sufficient capacity to cater for the drainage flow from the Application Site.

Mitigation measures are proposed during the construction period and to ensure that the existing drainage system within the site will not be affected during the construction stage.

## **APPENDIX A**

### **SITE LAYOUT PLAN**



## **APPENDIX B**

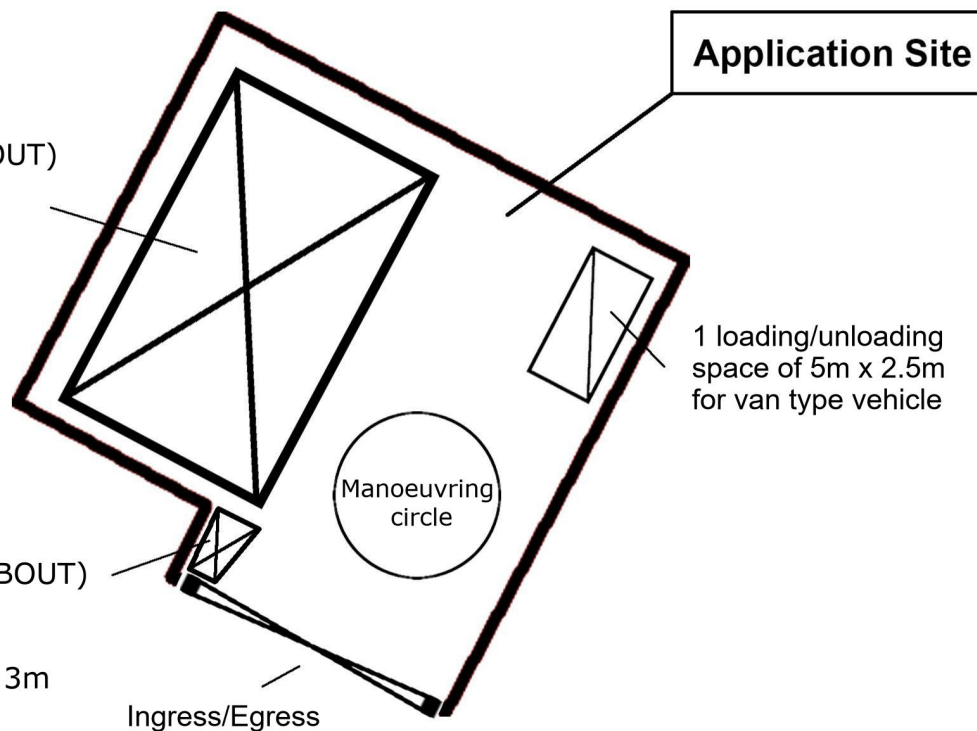
### **LAYOUT PLAN**

**Structure A**

Shop and Services  
 Covered Area: 115m<sup>2</sup> (ABOUT)  
 GFA: 230m<sup>2</sup> (ABOUT)  
 No. of storey: 2  
 Height: Not exceeding 7m

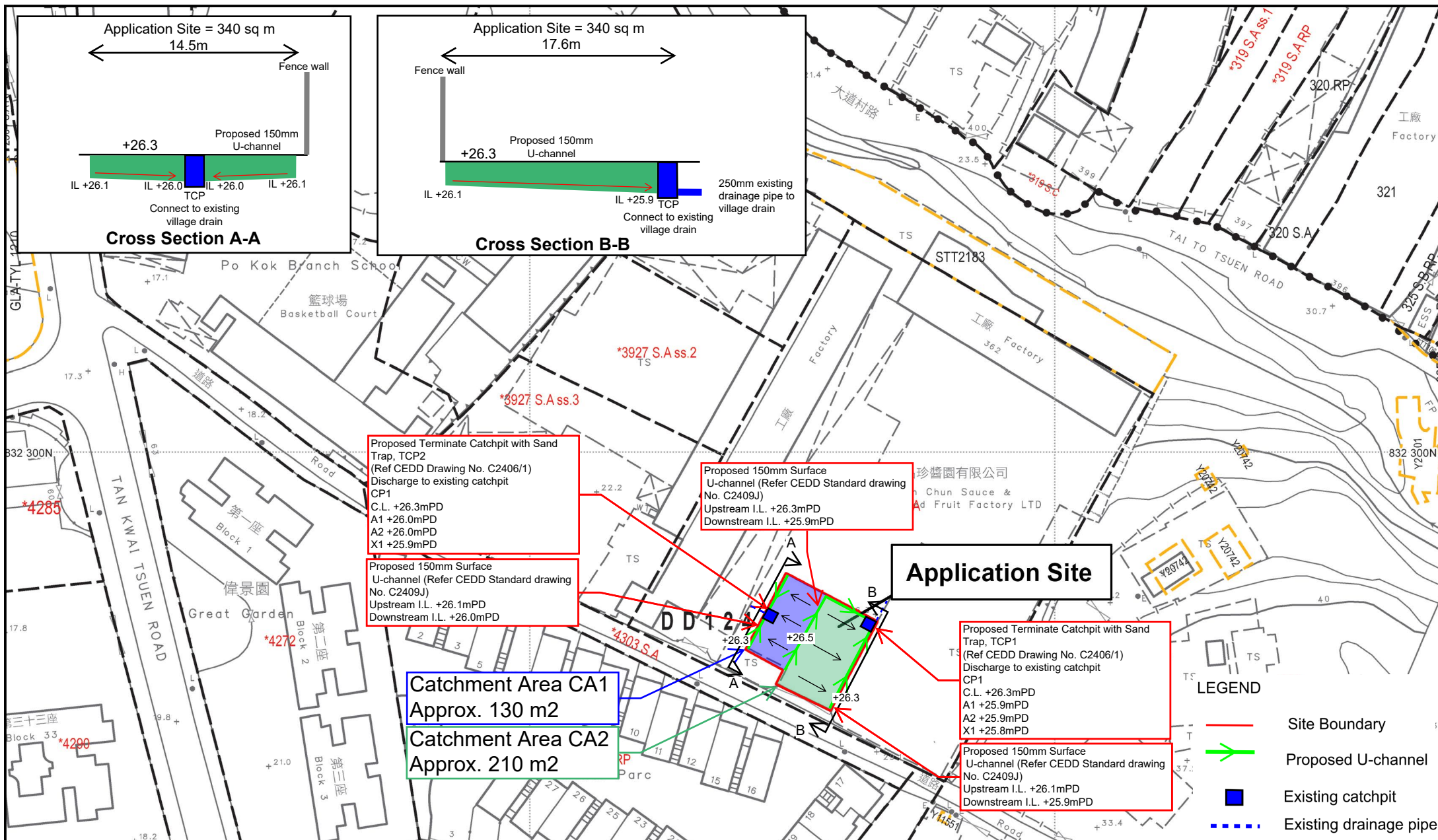
**Structure B**

Meter Room  
 Covered Area: 5m<sup>2</sup> (ABOUT)  
 GFA: 5m<sup>2</sup> (ABOUT)  
 No. of storey: 1  
 Height: Not exceeding 3m



## **APPENDIX C**

### **PROPOSED DRAINAGE PLAN**



Project 項目名稱:

Proposed Temporary Shop and Services for a Period of 3 Years at Lot 3927 S.A ss.4 (Part) in D.D. 124 ,Tan Kwai Tsuen, Hung Shui Kiu, Yuen Long, New Territories

Drawing Title 圖紙標題:

Layout Plan

Remarks 備註:

Drawing No. 圖號:

20260615



全堅土地規劃

## **APPENDIX D**

### **DESIGN CALCULATION OF THE PROPOSED DRAINAGE SYSTEM**

**PROJECT:** APPLICATION SITE OF PROPOSED TEMPORARY SHOP AND SERVICES FOR A PERIOD  
OF 3 YEARS AT LOTS 3927 S.A (PART) in D.D. 124, TAN KWAI TSUEN, HUNG SHUI KIU, YUEN LONG  
**JOB NO :** ALPL/TDM/004  
**TITLE :** Temporary Drainage Design Calculation

**Revision :** 0  
**Date :** 31-Mar-25  
**Prepare By:** JW

DETERMINE THE CATCHMENT OF AREA

CA1 = 130 (m<sup>2</sup>)  
CA2 = 210 (m<sup>2</sup>)

DETERMINE THE INLET TIME

| Section | A = Catchment of Area (m <sup>2</sup> ) |     | Level of remote point (mPD) | Level of inlet point (mPD) | L (m) | H (m/100m) | t <sub>e</sub> = Time of Natural flow (min) |
|---------|-----------------------------------------|-----|-----------------------------|----------------------------|-------|------------|---------------------------------------------|
| 1       | CA1                                     | 130 | 26.4                        | 26.3                       | 9     | 1.11       | 0.78                                        |
| 2       | CA2                                     | 210 | 26.4                        | 26.3                       | 11.8  | 0.85       | 1.03                                        |

Ref.: Brandsby Williams Equation

Note:  $t_e = \frac{0.14465 \cdot L}{H^{0.2} \cdot A^{0.1}}$   
H = average slope (m per 100m), measured along the line of natural flow, from the summit of the catchment to the point under consideration  
L = distance (on plan) measured on the line of natural flow between the summit and the point under consideration (m).

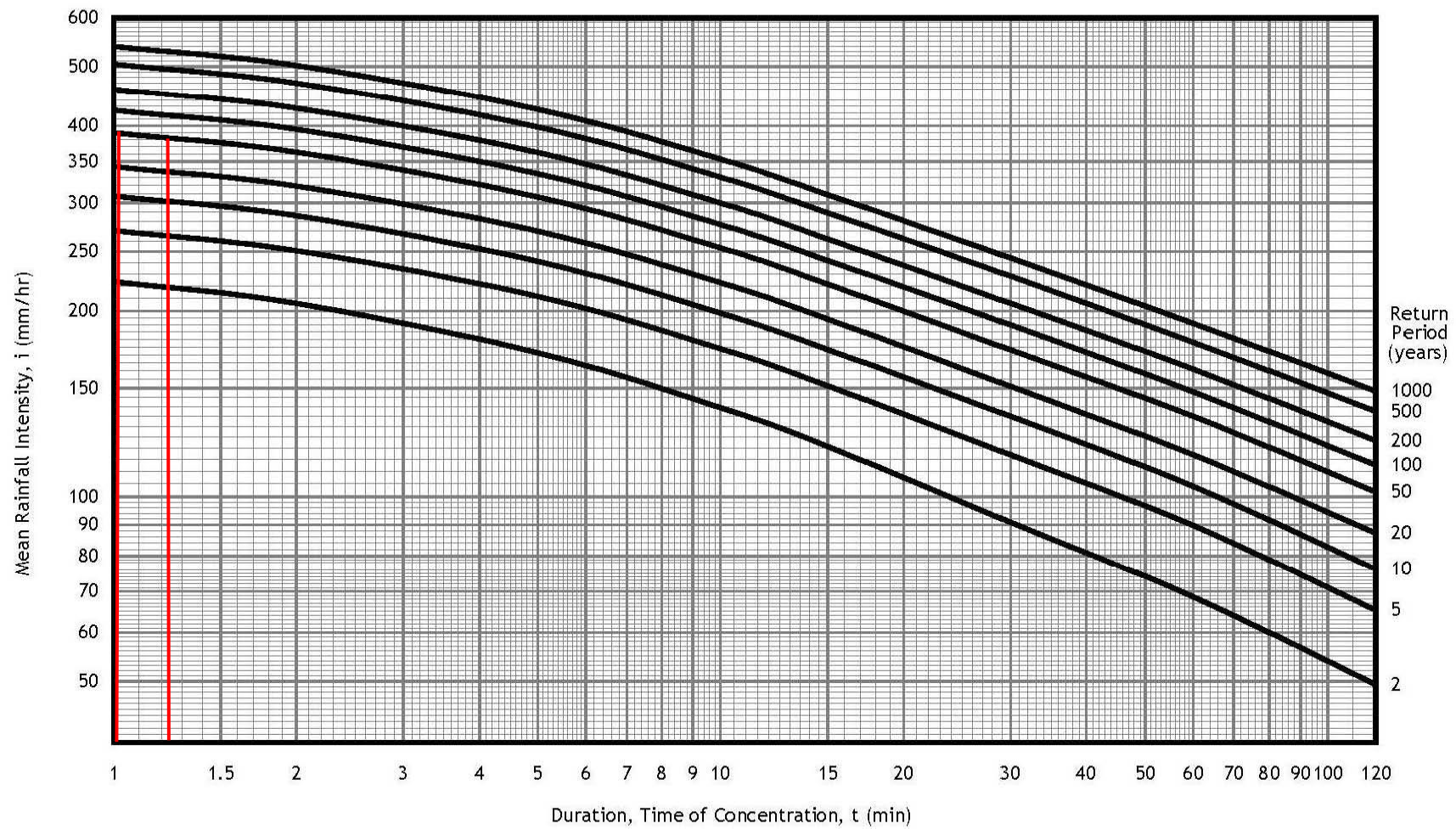
DETERMINE THE SIZE OF STEPPED / U-CHANNEL

| 1           | 2       | 3      | 4     | 5                | 6                           | 7                             | 8                            | 9                   | 10           | 11               | 12           | 13                   | 14                   | 15                   | 16        | 17             | 18     |
|-------------|---------|--------|-------|------------------|-----------------------------|-------------------------------|------------------------------|---------------------|--------------|------------------|--------------|----------------------|----------------------|----------------------|-----------|----------------|--------|
| Section No. | Ldr (m) | Lh (m) | L (m) | Gradient (1 in ) | Site area (m <sup>2</sup> ) | others area (m <sup>2</sup> ) | Total area (m <sup>2</sup> ) | A (m <sup>2</sup> ) | channel size | Capacity (l/min) | Assumed Flow | t <sub>f</sub> (min) | t <sub>c</sub> (min) | t <sub>e</sub> (min) | I (mm/hr) | Runoff (l/min) | Remark |
| 1           | 0.20    | 14.50  | 14.50 | 72.50            | 130.00                      | 0.00                          | 130.00                       | 130.00              | 150          | 1500             | 1.40         | 0.17                 | 0.78                 | 0.96                 | 390.00    | 760.5          | O.K.   |
| 2           | 0.30    | 17.60  | 17.60 | 58.67            | 210.00                      | 0.00                          | 210.00                       | 210.00              | 150          | 1600             | 1.50         | 0.20                 | 1.03                 | 1.23                 | 385.00    | 1212.8         | O.K.   |

CAPACITY > RUNOFF  
CAPACITY > RUNOFF

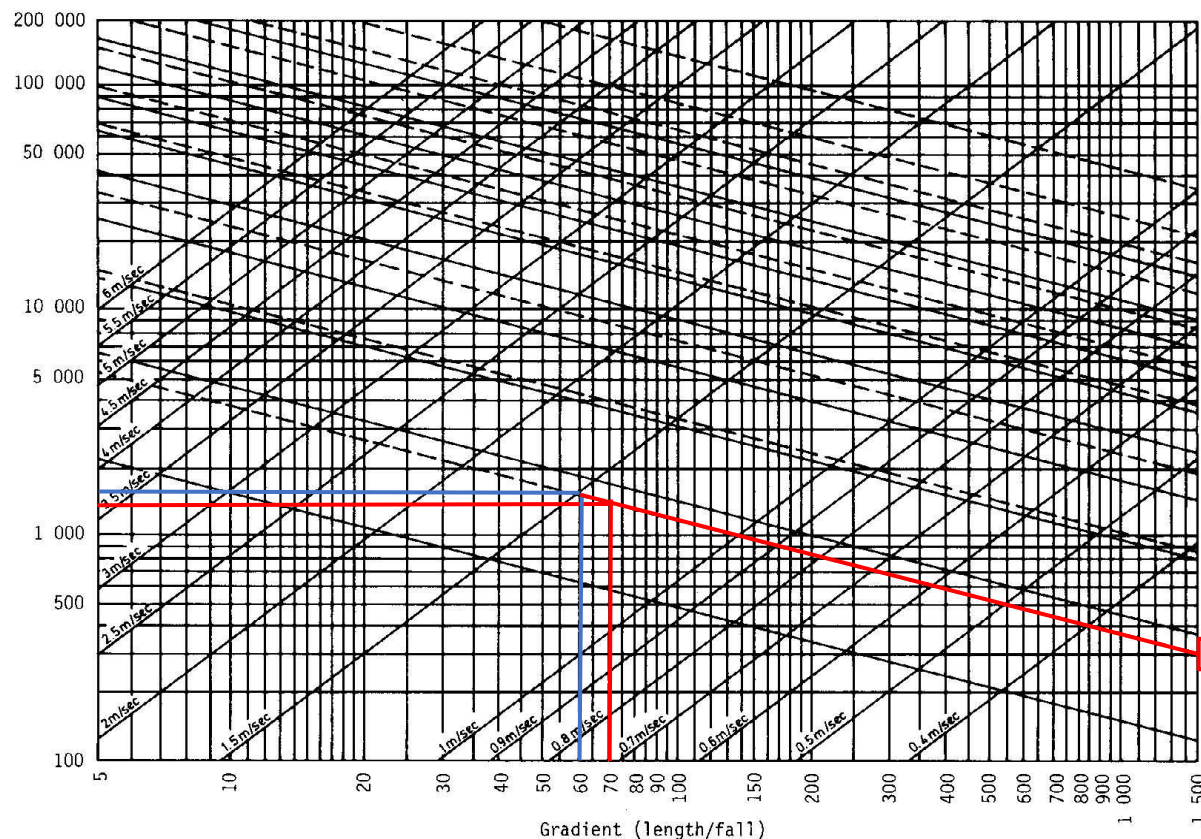
[Use new 150 U, so it is O.K.]  
[Use new 150 U, so it is O.K.]

Note: Ldr = Different level between U-channel section  
Gradient = 1 in (Lh/Ldr)  
Total area = Site area + others area  
A=Cumulative area = Total area + others section area  
Capacity = Refer to the extracted Figure 8.7 - Chart for the Rapid Design of Channels (Geotechnical Manual for Slopes P.253)  
Assumed flow velocity = Assumed velocity of runoff  
Actual Flow velocity = Refer to the extracted Figure 8.7 - Chart for the Rapid Design of Channels (Geotechnical Manual for Slopes P.253)  
t<sub>f</sub> = Flow time = L / assumed flow velocity  
t<sub>e</sub> = inlet time (time taken for flow from the remotest point to reach the most upstream point of the urban drainage system)  
t<sub>c</sub> = Time of concentration = t<sub>f</sub> + t<sub>e</sub> (Min. t<sub>c</sub> = 1 min. for conservative design)  
I = Intensity Refer to the extracted Figure 8.2 - Curves Showing Duration and Intensity of Rainfall in H.K. for Various Return Periods (Geotechnical Manual for Slopes P.248)  
K = Runoff coefficient = 0.9 refer to Character of Surface (refer DSD(2013))  
Design Return Period = 50 years  
Runoff = K.I.A./3600 (l/s) = K.I.A./60 (l/min)  
UC U-channel  
SC Stepped channel  
For section no. 1.00 , Actual Flow Velocity = 1.40 m/s < 4m/s, O.K.  
For section no. 2.00 , Actual Flow Velocity = 1.50 m/s < 4m/s, O.K.



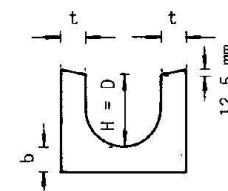
|     |     |
|-----|-----|
| 1.7 | 178 |
| 1.5 | 163 |

Capacities of Channels (litres per minute)

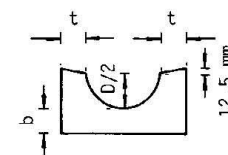


Size of channel (mm) shown by dotted solid line line

Channel dimensions



U-shaped channel (U)



Half-round channel (HR)

#### DESIGN METHOD USING CHART

##### (a) Normal channel Solution

1. Runoff
2. Gradient
3. Channel size
4. Velocity

Example :

1. Enter Runoff = 4 000 litre/min.
2. Enter Gradient = 1 in 40
3. Read channel required = 225 U or 300HR
4. Read velocity = 2.2 m/sec. (<4 m/sec. ∴ OK)

##### (b) Stepped channel Solution

2. Runoff
3. Channel size
4. Gradient
1. Velocity

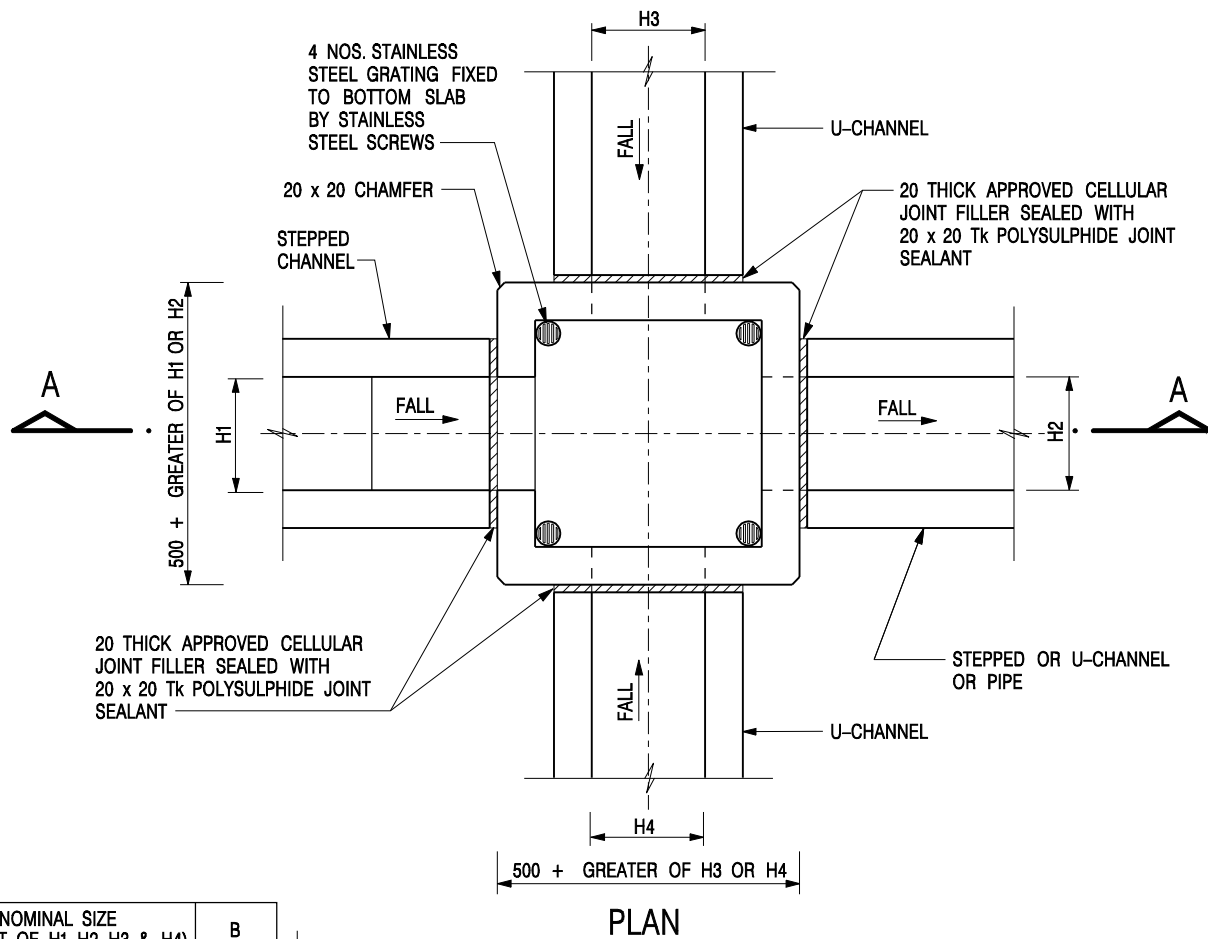
Example :

1. Enter Velocity = 5 m/sec.
2. Enter Runoff = 20 000 litre/min.
3. Read required channel size = 300U
4. Read required gradient = 1 in 14

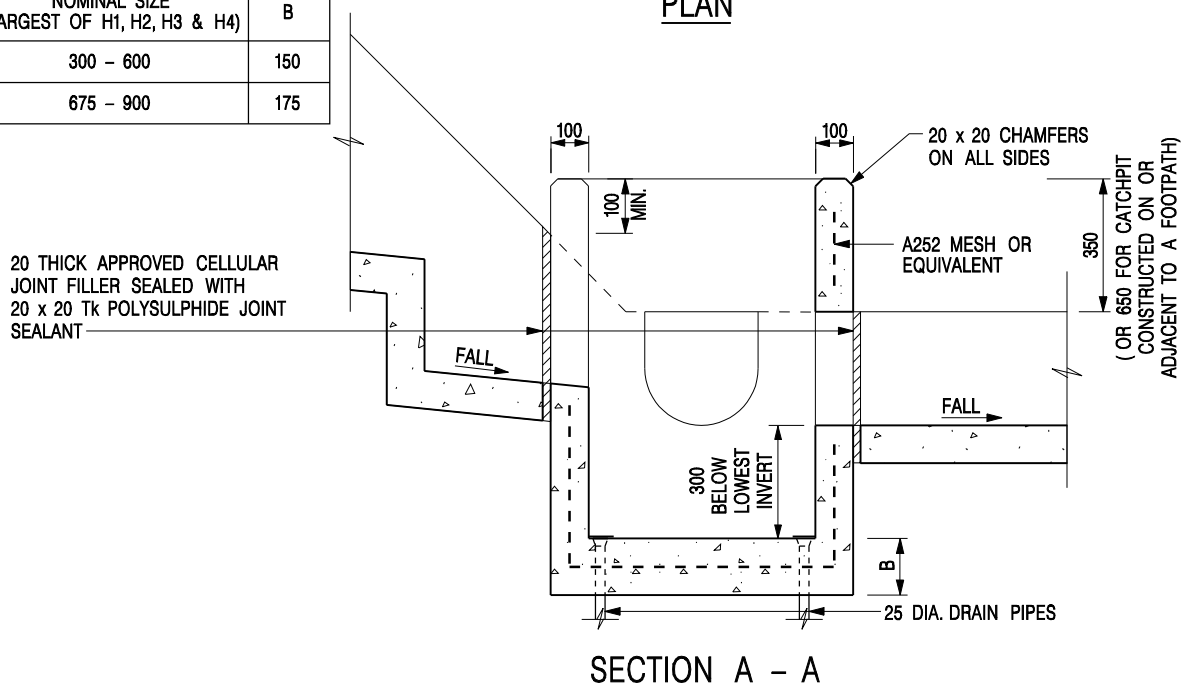
## **APPENDIX E**

### **TYPICAL STANDARD DRAWINGS OF U-CHANNEL AND CATCHPIT**

**(EXTRACTED FROM CEDD, FOR REFERENCE ONLY)**



| NOMINAL SIZE<br>(LARGEST OF H1, H2, H3 & H4) | B   |
|----------------------------------------------|-----|
| 300 - 600                                    | 150 |
| 675 - 900                                    | 175 |



**NOTES:**

1. ALL DIMENSIONS ARE IN MILLIMETRES.
2. REFER TO SHEET 2 FOR OTHER NOTES.

**CATCHPIT WITH TRAP**  
(SHEET 1 OF 2)

|      |                         |                 |         |
|------|-------------------------|-----------------|---------|
| -    | FORMER DRG. NO. C2406J. | Original Signed | 03.2015 |
| REF. | REVISION                | SIGNATURE       | DATE    |



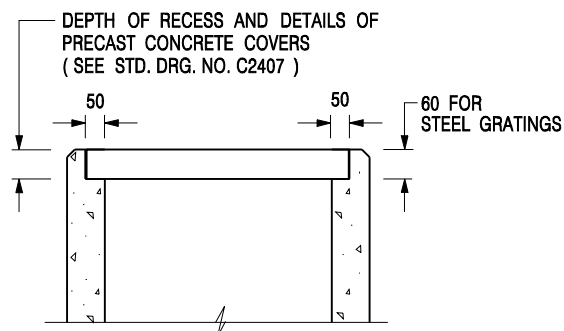
**CIVIL ENGINEERING AND  
DEVELOPMENT DEPARTMENT**

**SCALE** 1 : 20

**DATE** JAN 1991

**DRAWING NO.**

**C2406 /1**



### ALTERNATIVE TOP SECTION FOR PRECAST CONCRETE COVERS / GRATINGS

#### NOTES:

1. ALL DIMENSIONS ARE IN MILLIMETRES.
2. ALL CONCRETE SHALL BE GRADE 20 /20.
3. CONCRETE SURFACE FINISH SHALL BE CLASS U2 OR F2 AS APPROPRIATE.
4. FOR DETAILS OF JOINT, REFER TO STD. DRG. NO. C2413.
5. CONCRETE TO BE COLOURED AS SPECIFIED.
6. UNLESS REQUESTED BY THE MAINTENANCE PARTY AND AS DIRECTED BY THE ENGINEER, CATCHPIT WITH TRAP IS NORMALLY NOT PREFERRED DUE TO PONDING PROBLEM.
7. UPON THE REQUEST FROM MAINTENANCE PARTY, DRAIN PIPES AT CATCHPIT BASE CAN BE USED BUT THIS IS FOR CATCHPITS LOCATED AT SLOPE TOE ONLY AND AS DIRECTED BY THE ENGINEER.
8. FOR CATCHPITS CONSTRUCTED ON OR ADJACENT TO A FOOTPATH, STEEL GRATINGS ( SEE DETAIL 'A' ON STD. DRG. NO. C2405 /2 ) OR CONCRETE COVERS ( SEE STD. DRG. NO. C2407 ) SHALL BE PROVIDED AS DIRECTED BY THE ENGINEER.
9. IF INSTRUCTED BY THE ENGINEER, HANDRAILING ( SEE DETAIL 'J' ON STD. DRG. NO. C2405 /5; EXCEPT ON THE UPSLOPE SIDE ) IN LIEU OF STEEL GRATINGS OR CONCRETE COVERS CAN BE ACCEPTED AS AN ALTERNATIVE SAFETY MEASURE FOR CATCHPITS NOT ON A FOOTPATH NOR ADJACENT TO IT. TOP OF THE HANDRAILING SHALL BE 1 000 mm MIN. MEASURED FROM THE ADJACENT GROUND LEVEL.
10. MINIMUM INTERNAL CATCHPIT WIDTH SHALL BE 1 000 mm FOR CATCHPITS WITH A HEIGHT EXCEEDING 1 000 mm MEASURED FROM THE INVERT LEVEL TO THE ADJACENT GROUND LEVEL. AND, STEP IRONS ( SEE DSD STD. DRG. NO. DS1043 ) AT 300 c/c STAGGERED SHALL BE PROVIDED. THICKNESS OF CATCHPIT WALL FOR INSTALLATION OF STEP IRONS SHALL BE INCREASED TO 150 mm.
11. FOR RETROFITTING AN EXISTING CATCHPIT WITH STEEL GRATING, SEE DETAIL 'G' ON STD. DRG. NO. C2405 /4.
12. SUBJECT TO THE APPROVAL OF THE ENGINEER, OTHER MATERIALS CAN ALSO BE USED AS COVERS / GRATINGS.

|             |                         |                  |             |
|-------------|-------------------------|------------------|-------------|
| A           | MINOR AMENDMENT.        | Original Signed  | 04.2016     |
| -           | FORMER DRG. NO. C2406J. | Original Signed  | 03.2015     |
| <b>REF.</b> | <b>REVISION</b>         | <b>SIGNATURE</b> | <b>DATE</b> |

**CATCHPIT WITH TRAP  
(SHEET 2 OF 2)**



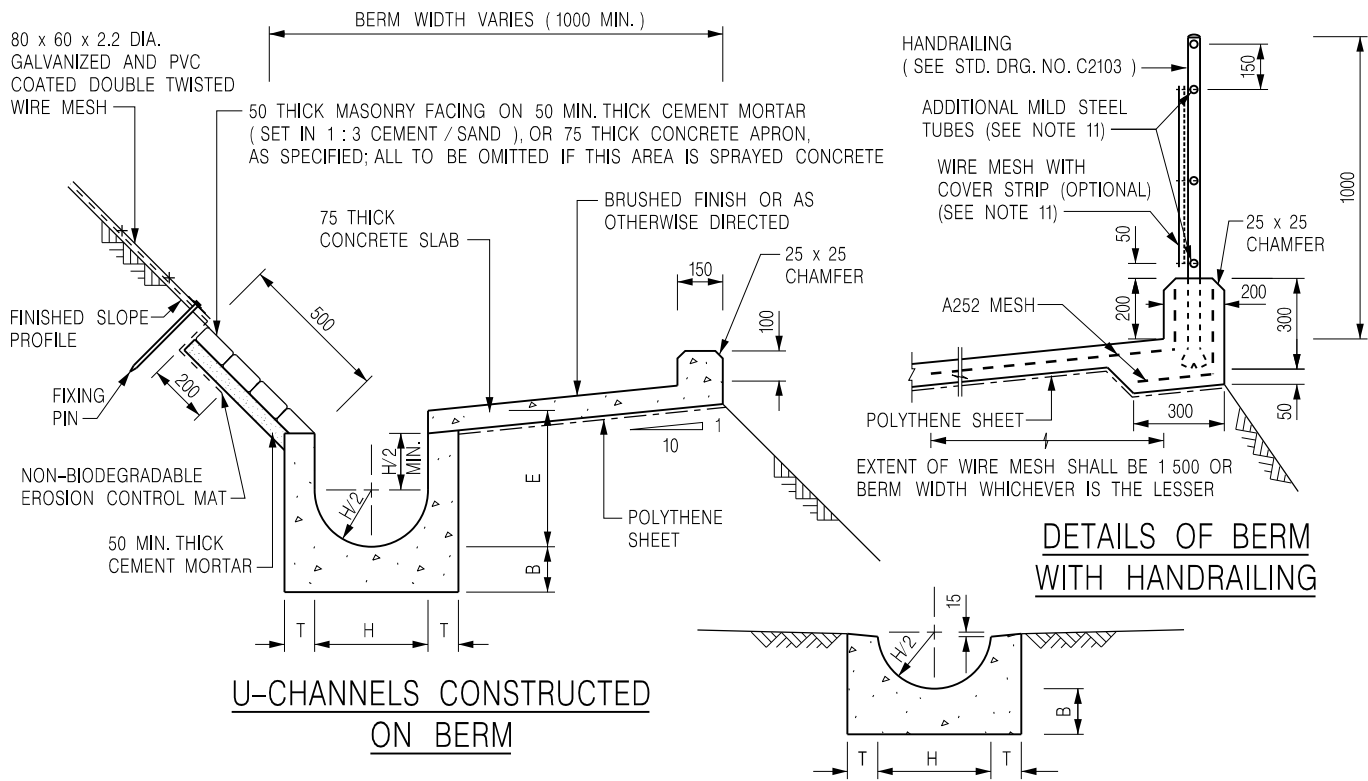
**CIVIL ENGINEERING AND  
DEVELOPMENT DEPARTMENT**

**SCALE** 1 : 20

**DATE** JAN 1991

**DRAWING NO.**

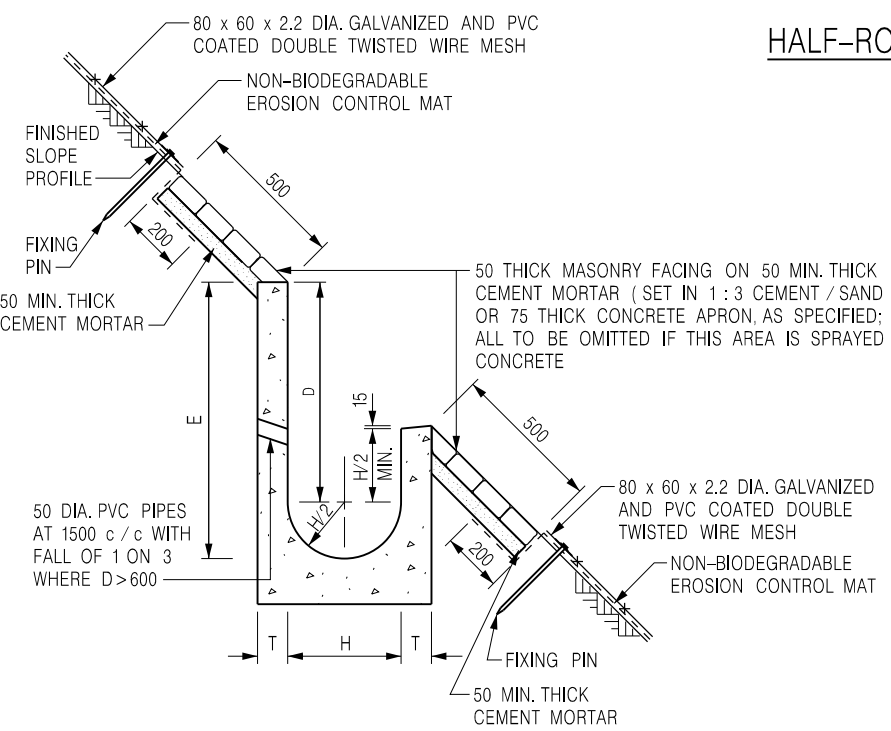
**C2406 /2A**



**U-CHANNELS CONSTRUCTED ON BERM**

**HALF-ROUND CHANNEL**

- NOTES:**
1. ALL DIMENSIONS ARE IN MILLIMETRES.
  2. ALL CONCRETE TO BE GRADE 20 / 20.
  3. CONCRETE SURFACE FINISH SHALL BE CLASS U2, F2 OR BRUSHED FINISH AS DIRECTED.
  4. SPACING OF EXPANSION JOINT IN CHANNELS, BERM SLABS AND APRONS TO BE 10 METRES MAXIMUM, SEE STD. DRG. NO. C2413 FOR DETAILS.
  5. JOINTS FOR CHANNELS, BERM SLABS, APRONS AND WALLS, ETC. TO BE ON THE SAME ALIGNMENT.
  6. FOR DIMENSIONS T, H, & B, SEE TABLE BELOW.
  7. BIODEGRADABLE EROSION CONTROL MAT IF REQUIRED, SEE STD. DRG. NO. C2511/E.
  8. CONCRETE TO BE COLOURED AS SPECIFIED.
  9. CONCRETE U-CHANNEL CAN BE CAST IN-SITU OR PRECAST CONCRETE SUBJECT TO THE ENGINEER'S AGREEMENT ON THE DETAILS.
  10. DETAILS OF EROSION CONTROL MAT AND WESH MESH ON BERM. (SEE STD. DRG. NO. C2511/E)
  11. THE WIRE MESH ON HANDRAILING IS OPTIONAL. THE COVER STRIP AND ADDITIONAL MILD STEEL TUBES ARE NEEDED ONLY IF WIRE MESH IS PROVIDED. (SEE STD. DRG. NO. C2103)



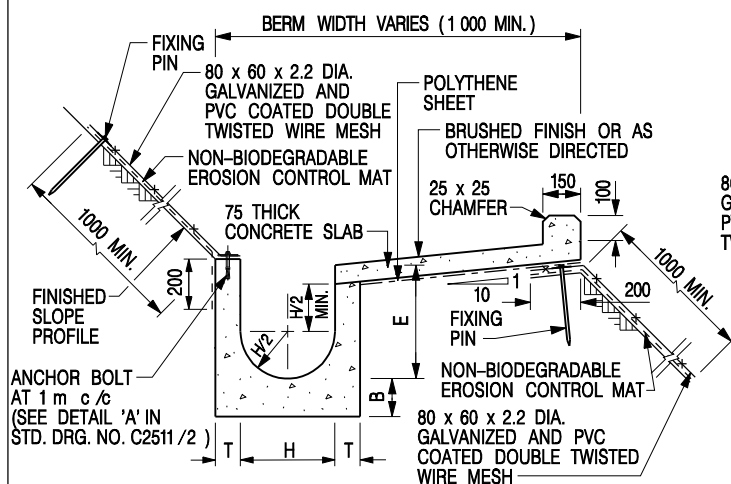
**U-CHANNELS NOT CONSTRUCTED ON BERM**

| NOMINAL SIZE<br>H | T   | B   | REINFORCEMENT                                   |
|-------------------|-----|-----|-------------------------------------------------|
| 300               | 80  | 100 | A252 MESH PLACED CENTRALLY AND T=100 WHEN E>650 |
| 375 - 600         | 100 | 150 |                                                 |
| 675 - 900         | 125 | 175 | A252 MESH PLACED CENTRALLY                      |

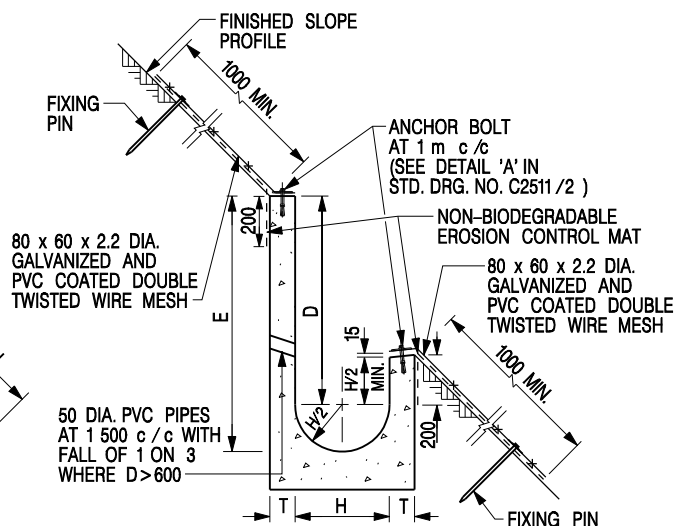
|      |                                      |                 |         |
|------|--------------------------------------|-----------------|---------|
| J    | DETAILS OF HANDRAILING AMENDED.      | Original Signed | 08.2024 |
| I    | MINOR AMENDMENT.                     | Original Signed | 07.2018 |
| H    | THICKNESS OF MASONRY FACING AMENDED. | Original Signed | 01.2005 |
| G    | MINOR AMENDMENT.                     | Original Signed | 01.2004 |
| F    | GENERAL REVISION.                    | Original Signed | 12.2002 |
| REF. | REVISION                             | SIGNATURE       | DATE    |

**DETAILS OF HALF-ROUND AND U-CHANNELS ( TYPE A – WITH MASONRY APRON )**

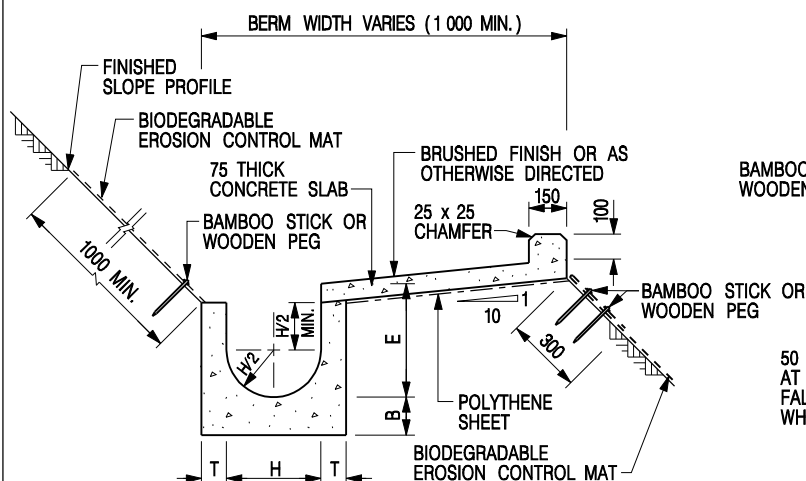
|                                                                                                                                         |                                     |
|-----------------------------------------------------------------------------------------------------------------------------------------|-------------------------------------|
|  <b>CIVIL ENGINEERING AND DEVELOPMENT DEPARTMENT</b> |                                     |
| <b>SCALE</b> 1 : 25<br><b>DATE</b> JAN 1991                                                                                             | <b>DRAWING NO.</b><br><b>C2409J</b> |



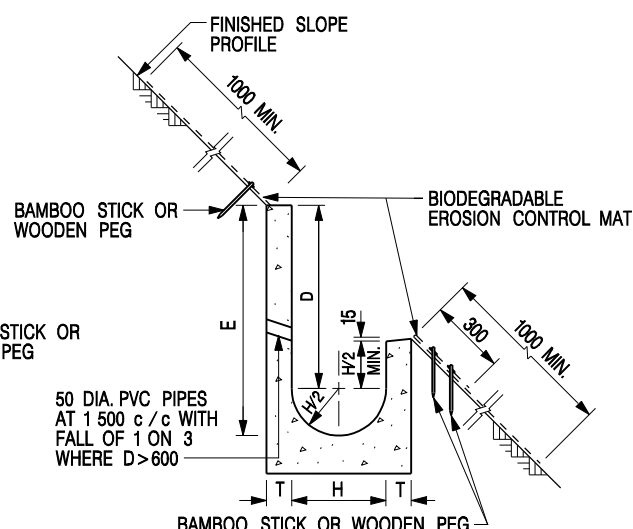
**U-CHANNELS CONSTRUCTED ON BERM  
WITH NON-BIODEGRADABLE  
EROSION CONTROL MAT**



**U-CHANNELS NOT CONSTRUCTED ON BERM  
WITH NON-BIODEGRADABLE  
EROSION CONTROL MAT**



**U-CHANNELS CONSTRUCTED ON BERM  
WITH BIODEGRADABLE  
EROSION CONTROL MAT**



**U-CHANNELS NOT CONSTRUCTED ON BERM  
WITH BIODEGRADABLE  
EROSION CONTROL MAT**

**NOTES:**

- ALL DIMENSIONS ARE IN MILLIMETRES.
- ALL CONCRETE TO BE GRADE 20 /20.
- CONCRETE SURFACE FINISH SHALL BE CLASS U2, F2 OR BRUSHED FINISH AS DIRECTED.
- SPACING OF EXPANSION JOINT IN CHANNELS, BERM SLABS AND APRONS TO BE 10 METRES MAXIMUM, SEE STD. DRG. NO. C2413 FOR DETAILS.
- JOINTS FOR CHANNELS, BERM SLABS, APRONS AND WALLS, ETC. TO BE ON THE SAME ALIGNMENT.
- FOR DIMENSIONS T, H, & B, SEE TABLE BELOW.
- FOR TYPICAL FIXING PIN DETAILS, SEE STD. DRG. NO. C2511/2.
- MINIMUM SIZE OF 25 x 50 x 300mm SHALL BE PROVIDED FOR WOODEN PEG.
- MINIMUM SIZE OF 10mm DIAMETER WITH 200mm LONG SHALL BE PROVIDED FOR BAMBOO STICK.
- THE FIXING DETAILS OF NON-BIODEGRADABLE AND BIODEGRADABLE EROSION CONTROL MATS ON EXISTING BERM SHALL REFER TO STD. DRG. NO. C2511/1.

| NOMINAL SIZE<br>H | T   | B   | REINFORCEMENT                                         |
|-------------------|-----|-----|-------------------------------------------------------|
| 300               | 80  | 100 | A252 MESH PLACED<br>CENTRALLY AND T=100<br>WHEN E>650 |
| 375 - 600         | 100 | 150 |                                                       |
| 675 - 900         | 125 | 175 | A252 MESH PLACED<br>CENTRALLY                         |

|      |                                                               |                 |         |
|------|---------------------------------------------------------------|-----------------|---------|
| I    | MINOR AMENDMENT.                                              | Original Signed | 07.2018 |
| H    | FIXING DETAILS OF BIODEGRADABLE<br>EROSION CONTROL MAT ADDED. | Original Signed | 12.2017 |
| G    | DIMENSION TABLE AMENDED.                                      | Original Signed | 01.2005 |
| F    | MINOR AMENDMENT.                                              | Original Signed | 01.2004 |
| E    | GENERAL REVISION.                                             | Original Signed | 12.2002 |
| D    | MINOR AMENDMENT.                                              | Original Signed | 08.2001 |
| C    | 150 x 100 UPSTAND ADDED AT BERM.                              | Original Signed | 6.99    |
| B    | MINOR AMENDMENT.                                              | Original Signed | 3.94    |
| A    | MINOR AMENDMENT.                                              | Original Signed | 10.92   |
| REF. | REVISION                                                      | SIGNATURE       | DATE    |

**DETAILS OF HALF-ROUND AND  
U-CHANNELS (TYPE B - WITH  
EROSION CONTROL MAT APRON)**



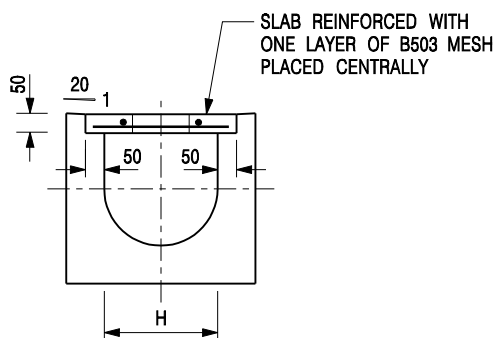
**CIVIL ENGINEERING AND  
DEVELOPMENT DEPARTMENT**

**SCALE** DIAGRAMMATIC

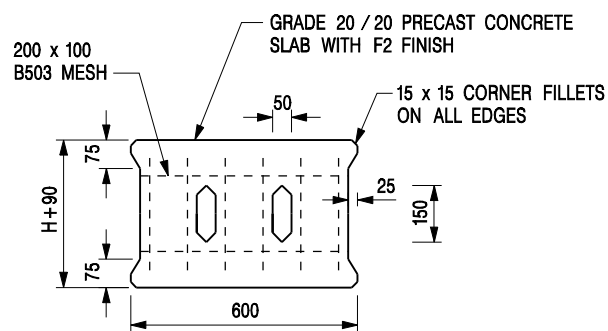
**DRAWING NO.**

**DATE** JAN 1991

**C24101**



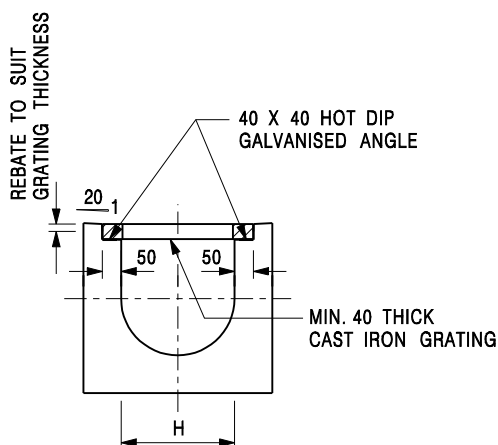
**TYPICAL SECTION**



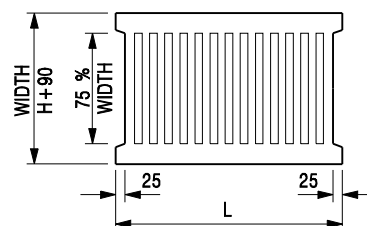
**PLAN OF SLAB**

## **U-CHANNELS WITH PRECAST CONCRETE SLABS**

(UP TO H OF 525)



**TYPICAL SECTION**



L = 600mm FOR H ≤ 375mm  
L = 400mm FOR H > 375mm

**CAST IRON GRATING**

(DIMENSIONS ARE FOR GUIDANCE ONLY, CONTRACTOR MAY SUBMIT EQUIVALENT TYPE)

## **U-CHANNEL WITH CAST IRON GRATING**

(UP TO H OF 525)

### **NOTES:**

- ALL DIMENSIONS ARE IN MILLIMETRES.
- H=NOMINAL CHANNEL SIZE.
- ALL CAST IRON FOR GRATINGS SHALL BE GRADE EN-GJL-150 COMPLYING WITH BS EN 1561.
- FOR COVERED CHANNELS TO BE HANDED OVER TO HIGHWAYS DEPARTMENT FOR MAINTENANCE, THE GRATING DETAILS SHALL FOLLOW THOSE AS SHOWN ON HyD STD. DRG. NO. H3156.

| E    | NOTES 3 & 4 AMENDED.           | Original Signed | 12.2014 |
|------|--------------------------------|-----------------|---------|
| D    | NOTE 4 ADDED.                  | Original Signed | 06.2008 |
| C    | MINOR AMENDMENT. NOTE 3 ADDED. | Original Signed | 12.2005 |
| B    | NAME OF DEPARTMENT AMENDED.    | Original Signed | 01.2005 |
| A    | CAST IRON GRATING AMENDED.     | Original Signed | 12.2002 |
| REF. | REVISION                       | SIGNATURE       | DATE    |

**COVER SLAB AND CAST IRON  
GRATING FOR CHANNELS**



**CIVIL ENGINEERING AND  
DEVELOPMENT DEPARTMENT**

**SCALE** 1 : 20

**DATE** JAN 1991

**DRAWING NO.**  
**C2412E**